# An Effective Water Depth Correction for Pressure-Based Wave Statistics on Rough Bathymetry Olavo B. Marques,<sup>a, b</sup> Falk Feddersen,<sup>a</sup> James MacMahan,<sup>b</sup> <sup>a</sup> Scripps Institution of Oceanography, University of California, San Diego, La Jolla, California

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ABSTRACT: Near-bottom pressure sensors are widely used to measure surface gravity waves. 7 Pressure spectra are usually converted to sea surface elevation spectra with a linear-theory transfer 8 function assuming constant depth. This methodology has been validated over smooth sandy 9 beaches, but not over complex bathymetry of coral reefs or rocky coasts. Bottom-mounted pressure 10 sensors co-located with wave buoys in 10-13 m water depth from a 5-week rocky-shorelines 11 experiment are used to quantify the error of pressure-based surface gravity wave statistics and 12 develop correction methods. The rough bathymetry has O(1) m vertical variability on O(1-10) m 13 horizontal scales, much shorter than the 90–40 m wavelength of sea-band (0.1-0.2 Hz). For 14 sensor stability, pressure sensors were deployed by divers in bathymetric lows. An effective 15 depth hypothesis is proposed where a spatially smoothed water depth provides more accurate 16 wave height statistics than the local depth at the pressure sensor. Pressure-based significant wave 17 height squared overestimates (as large as 21%) the direct wave buoy measurements, with elevated 18 biases in sea band, when using the pressure-sensor water depth in a bathymetric low. An optimal 19 depth correction, estimated by minimizing the wave height error, varies from 0.1-1.6 m. A 20 bathymetry smoothing scale of 13 m (1/3 of wavelength at 0.2 Hz) is found by minimizing the 21 smoothed bathymetry deviation relative to the optimal. The optimal and smoothed bathymetry 22 depth corrections are similar across locations and both corrections, using linear theory, significantly 23 reduce wave statistical errors. This suggests pressure sensor measurements can be effectively 24 corrected in regions with strong bathymetric variability over short length scales. 25

SIGNIFICANCE STATEMENT: The measurement of surface waves by bottom-mounted pressure sensors relies on wave theory formally derived for constant depth. We show that the constant depth assumption leads to systematic errors in wave statistics from observations over a rough, rocky bottom. By considering a spatially-smoothed bathymetry instead of the local water depth at the pressure sensor, the accuracy of wave energy density can be improved from 20% to 10%.

# **1. Introduction**

Pressure sensors are routinely used to describe surface gravity wave statistics such as wave 32 spectra, significant wave height, and wave energy flux, and are fundamental to observations of 33 wave transformation in the nearshore. Cross-shore arrays of pressure sensors provide gradients in 34 sea-swell wave statistics associated with wave shoaling and dissipation on sandy shore environments 35 (e.g., Thornton and Guza 1982, 1983; Raubenheimer et al. 1996; Herbers et al. 1999), coral reef 36 environments (e.g., Lowe et al. 2005; Monismith et al. 2015; Lentz et al. 2016; Rogers et al. 2016; 37 Acevedo-Ramirez et al. 2021; Sous et al. 2023), and rocky shores (Farrell et al. 2009; Poate et al. 38 2018; Gon et al. 2020; Lavaud et al. 2022). The energetics of surface gravity waves are important 39 for driving several processes in the nearshore, such as the circulation (e.g., MacMahan et al. 2006), 40 infragravity waves (e.g., Bertin et al. 2018), runup at the shoreline (e.g., Gomes da Silva et al. 41 2020), sediment transport on sandy beaches (e.g., Elfrink and Baldock 2002), and dispersal of 42 tracers (e.g., Moulton et al. 2023). Accurate estimates of surface gravity wave statistics from 43 pressure sensors are crucial for measuring how waves transform, drive currents, and induce mixing 44 between the surfzone and inner shelf. 45

Surface gravity wave statistics are typically estimated from pressure measurements using linear wave theory and assuming constant water depth, *h*. A transfer function *K* converts the observed pressure spectrum ( $S_p(f)$ , where *f* is frequency) to a surface elevation spectrum ( $S_\eta(f)$ ), i.e.,

$$S_{\eta}(f) = K^2 S_p(f), \tag{1}$$

where *K* is given by (e.g., Dean and Dalrymple 1991)

$$K = \frac{\cosh(kh)}{\cosh(kz_{\text{hab}})},\tag{2}$$

where  $z_{hab}$  is the height above the bottom for the pressure measurement, and *k* is the radian wavenumber derived from the linear-theory dispersion relationship,

$$\omega^2 = gk \tanh(kh),\tag{3}$$

where  $\omega$  is the radian wave frequency ( $\omega = 2\pi f$ ) and g is the gravitational acceleration. In practice, the water depth h is estimated from the mean pressure and knowing  $z_{hab}$ . In many nearshore applications, pressure sensors are deployed near the bed. Thus,  $z_{hab}$  is often small (1-10 cm) and  $\cosh(kz_{hab}) \approx 1$ . Similar transfer functions can be derived for constant depth from linear theory to relate horizontal and vertical velocity spectra to  $S_{\eta}$  (Herbers et al. 1992).

A well-known issue with this transformation is that K grows exponentially at large kh so that 57 pressure noise becomes amplified, and typically a high-frequency cut-off is applied to avoid 58 contamination of wave statistics (e.g., Raubenheimer et al. 1996). Validation of pressure-based 59 wave height statistics from (1)-(3) against statistics from direct measurements of the surface 60 elevation in the laboratory (Bishop and Donelan 1987) and in the field (Guza and Thornton 1980) 61 reported an accuracy within 10%, where the validation was performed over 0.1 < kh < 2, with 62 small enough  $K^2$  to prevent noise amplification. A few comparisons have been obtained between 63 directly measured  $S_n(f)$  and  $K^2S_p$  on the inner shelf. In a low-sloped sandy bay, co-located Spotter 64 (GPS-based) wave buoy and pressure sensor integrated within an ADCP (Acoustic Doppler Current 65 Profiler) in  $h \approx 7$  m have a good time-mean spectral comparison in the sea-swell (0.05–0.2 Hz) 66 band (Lancaster et al. 2021). Offshore of a low-sloped sandy beach in  $h \approx 10$  m, a comparison 67 between a pressure sensor and an acoustic surface tracker on an ADCP showed that linear theory 68 accurately estimated  $S_{\eta}$  out to at least  $kh \approx 1.5$  (Martins et al. 2021). Recently a comparison of 69 various wave buoys and a pressure sensor array in 8-m water depth, showed that the wave buoys 70 were consistent with the linear-theory transformed pressure measurements across the 0.07–0.25 Hz 71 band (Collins et al. 2023). The linear-theory transfer function (2) is derived under a constant h72 approximation. For smooth and weak bathymetric slope (i.e., bathymetry varying on scales longer 73 than a wavelength), this assumption works well both seaward of the surfzone (e.g., Herbers et al. 74 1992; Collins et al. 2023) where bathymetric slopes are typically < 0.01 and through the surfzone 75 (e.g., Thornton and Guza 1983; Herbers et al. 1999) where bathymetric slopes are generally < 0.04. 76 Wave nonlinearity is not incorporated in (1)-(3), and increasingly nonlinear waves modify the 77 relationship between near-bed pressure and sea-surface elevation. A weakly nonlinear and weakly 78 dispersive (small kh) method can reproduce the sea-surface of a soliton from bottom pressure 79 (Bonneton and Lannes 2017) and wave time series for just offshore of the surfzone (Bonneton et al. 80 2018). For O(1) kh where triads are not resonant, the relationship between  $S_n$  and  $S_p$  can change 81

as a certain fraction of the wave energy at a particular frequency is bound (e.g., Hasselmann 82 1962). However, in  $\approx 7$  m depth, the fraction of bound energy at f < 0.2 Hz is generally small 83 even for large waves (Herbers et al. 1992), and the relationship between pressure and velocity is 84 well predicted by linear wave theory (Herbers et al. 1992). This relationship is so consistent even 85 within the surfzone that it is used as a method of quality controlling current meter data (Elgar 86 et al. 2001). For weakly dispersive waves, large waves can also change the dispersion relationship 87 through amplitude dispersion which was detectable in the field (Herbers et al. 2002) and laboratory 88 (Martins et al. 2021). 89

Linear theory also neglects the velocity-squared terms in the Bernoulli equation, which can be significant for estimating wave setdown and setup (Raubenheimer et al. 2001). However, for realistic conditions, this term contributes 2.5 cm root-mean-square to hydrostatic pressure (Lentz and Raubenheimer 1999) and is thus generally negligible for estimating wave properties.

In contrast with sandy beaches, coral reefs and rocky shores support large multiscale bathymetric 94 variability at scales much shorter than the sea-swell wavelengths (i.e., large slopes and slope 95 variability), and the constant h assumption in (1)-(3) is questionable. For example, coral reef 96 bathymetry has steep fore reefs, gently sloping flat reefs, and spur-and-groove formations, all of 97 which can have O(1) m depth changes over O(1) m horizontal distance (e.g., Monismith 2007; 98 Davis et al. 2021), scales much shorter than the O(10-100) m wavelength of sea-swell waves. 99 Despite complex bathymetry, wave statistics are often estimated by applying linear wave theory 100 to pressure sensor data, taking h as the depth calculated from the data (e.g., Monismith et al. 101 2015). Similarly, wave height estimates from pressure sensors have also been made over rocky 102 bathymetry, which may have O(1) m variability in h over horizontal scales much shorter than 103 sea-swell wavelengths (Farrell et al. 2009; Poate et al. 2018; Gon et al. 2020; Lavaud et al. 2022). 104 No validation of pressure-derived wave statistics has been performed on coral reefs or rocky shores. 105

Assuming a constant h approximation with (2)-(3) can be used in rough complex bathymetric 106 regions to derive wave statistics, it is unclear that the local pressure-sensor estimated h is the 107 appropriate choice. For wavelengths and water depths with small kh (e.g., Lentz et al. 2016), 108 waves are largely hydrostatic, K is approximately 1 throughout the water column, and the choice 109 of h may not be important. However, in regions with O(1) kh and rough bathymetry, the transfer 110 function is likely sensitive to the depth, which would affect wave statistics. Accurate surface 111 gravity wave statistics are particularly important for spatial instrument arrays where gradients of 112 wave statistics are taken across horizontal scales of O(10-100) m. Gradients of wave energy 113 flux derived from pressure sensors show larger wave bottom friction dissipation over coral reefs 114 or rocky shores than on sandy beaches (e.g., Lowe et al. 2005; Gon et al. 2020). Large bottom 115 friction dissipation has been observed (Lowe et al. 2005; Gon et al. 2020) at large water depth, 116

where depth-limited wave breaking is negligible, but errors in *K* for large kh could be significant. Therefore, if the constant depth assumption underlying (1)-(3) lead to substantial errors in the surface elevation spectrum, the contamination not only extends to wave height and energy flux but also to wave dissipation estimates across the array.

Here we use bottom-mounted pressure sensors with co-located wave buoys to address the accu-121 racy of linear wave theory to estimate wave heights from pressure data over complex and rough 122 bathymetry in approximately 10 m water depth. Observations are from a 5-week experiment that 123 was carried out in the Monterey Peninsula (California, USA) as part of ROXSI (ROcky Shore: 124 eXperiment and SImulations). The instrument array and bathymetry are described in Section 2. 125 The accuracy of (1)-(3) with a local water depth is quantified in Section 3. In Section 4, we pro-126 pose and test an effective depth hypothesis, where the depth from a spatially-smoothed bathymetry 127 results in more accurate wave statistics than the local depth from a pressure sensor. Comparisons 128 with a sandy inner-shelf, the implications of the effective depth, and application to coral reefs are 129 discussed in Section 5. A summary is presented in Section 6. 130

### 131 **2. Methods**

# <sup>132</sup> *a. Field site and bathymetry*

The first ROXSI (ROcky shores: eXperiments and SImulations) field experiment was carried out off China Rock, Pebble Beach, CA, USA during June-July 2022 (Fig. 1). The goal of ROXSI is to study how rough rocky bathymetry impact waves and circulation in the nearshore. The shoreline at China Rock (Fig. 1a) and most of the bathymetry (Fig. 1b) is composed of large rocks.

Multiple datasets were combined to map the bathymetry, as in contrast to sandy shores, rocky 137 morphology only changes on geological timescales. Multibeam bathymetry gridded at 2 m resolu-138 tion for water depths greater than  $\approx 10$  m is available from the California State University, Monterey 139 Bay. Shallower bathymetry was measured with a bathymetric lidar by the Joint Airborne Lidar 140 Bathymetry Technical Center of Expertise (JALBTCX). Lidar returns have an irregular distribu-141 tion, with a typical resolution between 0.5-2 m. The JALBTCX dataset covers most of the region 142 with water depth < 10 m. Bathymetry was also measured from surveying system on a Rotinor 143 DiveJet underwater scooter. Flotation was added to the DiveJet, which is operated at the surface 144 by one person. A frame was mounted in front of the DiveJet to hold a survey-grade GPS above a 145 downward-looking Nortek Signature1000 Acoustic Doppler Current Profiler (ADCP). The Nortek 146 Signature1000 has an echosounder that was programmed to sample at 4 Hz. Subaerial topography 147 is available from the National Oceanic and Atmospheric Administration (NOAA). Elevations rel-148

- ative to mean sea level (z) from the combined datasets were gridded to 2 m horizontal resolution
- (Fig. 1b). A local (x, y) coordinate system is defined where -x is offshore directed to 285°N.



FIG. 1. (a) Image of the China Rock (Pebble Beach, CA, USA) shoreline taken at low tide, where rocks can 151 be a few meters tall. (b) Instrument array (circles) over the rough rocky bathymetry off China Rock. An array 152 of eight Smart Moorings (yellow dots), co-located wave buoys, and pressure sensors, was deployed at a depth 153 of  $\approx 10$  m. We denote the Smart mooring locations as S1 to S8 going from south to north. Colors in (b) show 154 the 2-m gridded elevation relative to mean sea level elevation with the 10 m isobaths contoured. Location from 155 where the photo in (a) was taken is denoted by the magenta triangle. (c) Ungridded perturbation depth relative to 156 the depth of the S3 pressure sensor (i.e.  $-(h - \overline{h}_p)$ ), where positive (red) and negative (blue) indicate shallower 157 and deeper depths than at the pressure sensor (yellow), respectively. Maps in (b) and (c) are shown in a local 158 cross- and alongshore (x, y) coordinate system. 159

The shoreline and bathymetry at China Rock have variability at a wide range of scales (Fig. 1). 160 On horizontal scales of hundreds of meters, the shoreline has small headlands and embayments 161 spaced by 100-200 m. The bathymetry has a moderate (1:40) cross-shore slope. Rocky formations 162 lead to large seafloor roughness on vertical scales of O(1-10) m (Fig. 1c). For example, the 163 standard deviation of z within 5 by 5 m squares has a median of 0.5 m across the study site. The 164 difference between the maximum and minimum in each square, which is a better representation 165 of the height of larger rocks, has a median of 2 m (consistent with the photo in Fig. 1a and the 166 perturbation depth in Fig. 1c). In addition to areas with large bottom roughness, rock aggregates 167 are mingled with patches of sand, where the bathymetry is smoother (e.g. around x = -600 m and 168 y = 0 m in Fig. 1b). 169

### 170 b. Instruments and Data Processing

A 54-instrument array was deployed from June 17th to July 20th 2022 to measure wave trans-171 formation over the rocky bathymetry off China Rock. Instruments measuring surface gravity 172 waves included Sofar Spotter wave buoys that measured the sea surface directly (Herbers et al. 173 2012; Raghukumar et al. 2019), Nortek Acoustic Doppler Current Profilers (ADCP), and bottom-174 mounted RBR Coda and soloD pressure sensors (blue circles in Fig. 1b). Here, we will focus on 175 an alongshore array around the 10 m isobath of 8 Sofar Smart Moorings (yellow circles in Fig. 1), 176 which have co-located pressure and sea-surface elevation measurements from bottom-mounted 177 RBR Coda pressure sensors cabled to Spotter wave buoys. The Spotter provides horizontal and 178 vertical surface displacements at frequencies 0.05 to 2.5 Hz. The co-located pressure sensors, 179 sampling at 2 Hz, were deployed in bathymetric lows on weighted plates at a height above the 180 local rough rocky bathymetry  $z_{hab} = 0.13$  m (Fig. 1c). Given the large bottom roughness, the 181 water depth in a pressure sensor's vicinity (i.e., at 10 m horizontal scale) can be a few meters 182 shallower. Pressure in units of Pa is converted to units of meters by normalization with  $\rho_0 g$  where 183  $\rho_0 = 1025 \text{ kg m}^{-3}$  and  $g = 9.8 \text{ ms}^{-2}$ . Hourly-averaged atmospheric pressure  $P_{\text{atm}}$  was measured at 184 a NOAA pressure gauge in Monterey Harbor ( $\approx 6$  km from our site). A 3 cm offset was subtracted 185 from  $P_{\rm atm}$  based on a comparison to our pressure sensors when exposed in the intertidal zone. The 186 hourly-averaged water depth  $h_p$  is given by 187

$$h_p = \frac{P - P_{\text{atm}}}{\rho_0 g} + z_{\text{hab}},\tag{4}$$

where P represents the hourly-averaged pressure p.

Hourly pressure spectra  $S_p$  were computed using 120 s-long segments (frequency resolution  $\approx 0.008$  Hz) that were tapered with a Hanning window and with 50% overlap yielding 118 degrees



FIG. 2. Co-located pressure-based  $(H_p^2)$  versus Spotter wave-buoy-based  $(H_{sp}^2)$  significant wave height squared at the 8 Smart Mooring locations (Fig. 1b). The mean squared error (6) and regression slope with its 95% confidence limit are shown at each location. From the regression slopes,  $H_p^2$  consistently overestimates  $H_{sp}^2$ .

<sup>191</sup> of freedom. Surface elevation wave spectra  $S_{\eta}$  from the Spotter were similarly computed. The <sup>192</sup> standard approach to compute  $S_{\eta}$  from  $S_p$  is to use the local depth  $h_p$  to calculate wavenumbers <sup>193</sup> *k* through the linear dispersion relationship (3) and the transfer function *K* (2). This approach <sup>194</sup> assumes constant depth. The significant wave height can then be computed from either Spotter <sup>195</sup> ( $H_{sp}$ ) or pressure ( $H_p$ ) measurements as

$$H \equiv 4\sqrt{\int S_{\eta} \,\mathrm{d}f}.\tag{5}$$

Throughout this paper, we compute significant wave height H between 0.1 and 0.2 Hz. For the 196 range in time-mean water depths at instrument locations (9.7 <  $\bar{h}_p$  < 13.6 m), the frequency range 197 where H is computed corresponds to wavelengths between 36 and 105 m and kh between 0.7 198 and 2.2. The frequency band 0.1 < f < 0.2 Hz includes the surface wave peak periods for most 199 of the experiment and has negligible contamination from pressure noise amplified by K at high 200 frequencies. Based on a  $S_p$  noise floor of  $5 \times 10^{-6}$  m<sup>2</sup> Hz<sup>-1</sup> and a water depth of 13.6 m, the error 201 in  $H^2$  is less than 1 cm<sup>2</sup>. In the depth range of 10–13 m, pressure-sensor-based estimates of  $S_{\eta}$  are 202 overwhelmed by noise at frequencies higher than 0.3 Hz. 203

# <sup>207</sup> **3.** Accuracy of transfer function using $h_p$

Significant wave height  $H_p$ , estimated from pressure and the pressure-estimated local water depth  $h_p$ , is known to be accurate on low-sloped sandy coastlines (e.g., Guza and Thornton 1980), as long as kh is not too large such that sensor noise is amplified. However, the rocky bathymetry at our site has large vertical variability on horizontal scales of O(1-10) m, which are shorter than the wavelength of sea and swell surface gravity waves (Fig. 1c). Thus, it is unclear whether  $h_p$  leads to reliable estimates of  $H_p$ . Since the Smart Moorings provide co-located pressure and surface elevation measurements, we can assess the accuracy  $H_p$ .

At the eight Smart Mooring locations, the Spotter significant wave height  $H_{sp}$  varied from 0.2 to 2 m (corresponding to a range of  $H_{sp}^2$  between 0.04 and 4 m<sup>2</sup>, Fig. 2) largely on synoptic and diurnal time scales. Here, we focus on  $H^2$  as it directly relates to wave energy. Along the array, the time-mean  $H_{sp}^2$  varied between 0.65 and 0.81 m<sup>2</sup>, with no consistent alongshore pattern. The observed  $H_p^2$  (using  $h_p$ ) is in overall reasonable agreement with  $H_{sp}^2$ , but  $H_p^2$  is biased high at most locations (Fig. 2). The accuracy of  $H_p^2$  relative to  $H_{sp}^2$  is quantified with the correlation coefficient, the linear regression slope, and the mean-squared error, i.e.

$$\epsilon_0^2 = \overline{\left(H_p - H_{sp}\right)^2},\tag{6}$$

where  $\overline{(\ldots)}$  is a time average over the experiment duration (33 days), and which is proportional to the wave energy density. Both bias and random noise affect  $\epsilon_0^2$ .

Along the array,  $H_p^2$  is consistently biased high relative to  $H_{sp}^2$  (Fig. 2), even though the squared correlation between the two is high at all locations ( $r^2 > 0.94$ , not shown). The regression slopes vary from 1.21 at S2 to 0.99 at S4 with an average of 1.09. The southernmost locations have the highest slopes. The regression slope is significantly above unity at 7 out of 8 locations, whereas the underestimate at S4 is statistically insignificant. Larger  $\epsilon_0^2$  is primarily associated with larger regression slopes (e.g., S2).

We next compare the time-average (over the experiment duration) of the Spotter wave spectra  $\overline{S_{\eta}}$ 234 to the pressure-sensor wave spectra  $\overline{K^2(h_p) S_p}$  at location S3, which had a large but not the largest 235 overestimate of  $H_{sp}^2$  (Fig. 3). The mismatch between  $\overline{K^2 S_p}$  and  $\overline{S_n}$  is frequency-dependent. In 236 the swell band (f < 0.1 Hz), the two spectra are largely similar as the kh are relatively small and 237  $K^2 \leq 1.6$ . However, in the sea-band (0.1 < f < 0.2 Hz),  $\overline{K^2(h_p) S_p}$  is consistently elevated over 238  $\overline{S}_{\eta}$ , where the ratio between their sea-band integrated spectra is 1.17. Therefore, the overestimated 239  $H_p$  is due to sea-band waves (and not swell) as in the  $\approx 10$  m depth of the Smart Mooring array, the 240 sea-band has kh > 0.7 corresponding to rapidly growing  $K^2 = 8.3$  at f = 0.2 Hz. The overestimated 241  $H_p^2$  and  $K^2S_p$  will lead to overestimated wave energy, wave energy fluxes, and radiation stress, 242



FIG. 3. Time-averaged Spotter sea-surface elevation spectra  $\overline{S}_{\eta}$  (black) and pressure-sensor estimated  $\overline{K^2S_p}$ versus frequency at location S3. The time-averaged pressure sensor measured depth is  $\bar{h}_p = 9.7$  m. The top axis shows nondimensional  $k\bar{h}_p$  corresponding to the frequency axis and the  $\bar{h}_p$  using the linear dispersion relationship (3).

which all depend on the sea-surface elevation spectrum. We next explore the cause of the bias
between the pressure sensor and Spotter and how to correct the bias.

# **4.** Correction of wave height estimates over complex bathymetry

The overprediction with high correlation of wave heights with pressure sensors located in bathy-246 metric lows with dispersion relationship and transfer function evaluated at  $h_p$ , suggests that linear 247 wave theory is largely appropriate but that using  $h_p$  leads to errors. Linear wave theory (3) and 248 (2) is derived for a constant depth and is valid for low slopes (i.e., slowly varying bathymetry). 249 However, here the bathymetry has large variability on horizontal scales much shorter than sea and 250 swell wavelengths (Fig. 1c). If  $K(h_p)$  was an accurate transfer function, this would suggest that 251 surface waves would be adjusting over short horizontal distances to sharp bathymetric changes, 252 contradicting linear theory. However, if surface gravity waves are instead only responding to water 253 depth changes at some longer spatial scales, then simply an appropriate effective water depth  $h_{\rm eff}$ , 254 different than  $h_p$  is required for use in linear theory. 255

Our hypothesis, denoted the effective depth hypothesis, is wave statistics can be corrected by replacing  $h_p$  with an effective depth  $h_{\text{eff}}$  from the spatially smoothed bathymetry (Fig. 4). For sensors deployed in bathymetric lows,  $h_{\text{eff}} < h_p$  which leads to  $K(h_{\text{eff}}) < K(h_p)$  and thereby reducing the overestimation in  $K^2S_p$  and  $H_p^2$ . Therefore, using  $h_{\text{eff}}$  instead of  $h_p$  could reduce the observed  $H_p^2$  and time-mean spectra errors. However, it is unclear a priori what the relevant



FIG. 4. Schematic of the rough bathymetry with a pressure sensor in a bathymetric low. Water depth is defined as positive, where  $h_p$  is the local depth at the pressure sensor and the effective depth  $h_{\text{eff}}$  is a spatially smoothed bathymetry. For a pressure sensor in a bathymetric low,  $\delta h < 0$ .

spatial scale for bathymetric-smoothing is and how to calculate a depth correction  $\delta h$  such that  $h_{\text{eff}} = h_p + \delta h$ . Using the bathymetry and co-located measurements of pressure and sea-surface elevation from the Smart Mooring array, the effective depth hypothesis can be tested. We first calculate an effective depth correction using only the wave observations and then compare it to bathymetry smoothed at different spatial scales.

# *a. Effective depth from observations*

To determine an optimal water depth correction  $\delta h_{\text{opt}}$ , we find the depth correction that minimizes the error between  $H_p$  and  $H_{\text{sp}}$ . The error  $\epsilon^2(\delta h)$  is defined similar to (6),

$$\epsilon^{2}(\delta h) = \overline{\left(H_{\rm p}(h_{p} + \delta h) - H_{\rm sp}\right)^{2}},\tag{7}$$

where  $H_p(h_p + \delta h)$  is based on  $K^2(h_p + \delta h)S_p$  integrated between 0.1 and 0.2 Hz, and the depth change also modifies the estimated wavenumbers k in the linear dispersion relationship (3). At each location, we compute  $\epsilon^2(\delta h)$  where  $\delta h$  is varied from -3 m to 0 m at 0.1 m intervals. The optimal water depth correction  $\delta h_{opt}$  equals  $\delta h$  that minimizes (7). Posterior estimates on the uncertainty of  $\delta h_{opt}$  are estimated assuming  $(\epsilon^2(\delta h) - \epsilon^2(\delta h_{opt}))/\epsilon^2(\delta h_{opt})$  is a Gaussian random variable.

For example,  $\epsilon^2(\delta h)$  at location S3 is shown in Fig. 5. For no depth correction ( $\delta h = 0$ ),  $\epsilon^2(\delta h)$ is equivalent to  $\epsilon_0^2 = 80 \text{ cm}^2$  (Fig. 2, S3). The error  $\epsilon^2$  is a parabola with  $\delta h$  and the optimal  $\delta h_{\text{opt}} = -1.5 \text{ m}$  minimizes  $\epsilon^2(\delta h)$  to 20 cm<sup>2</sup> reducing the mean squared error to one quarter of  $\epsilon_0^2$ . Negative  $\delta h_{\text{opt}}$  is consistent with pressure sensors deployed in bathymetric lows (Fig. 1c) and the effective depth hypothesis (Fig. 4), i.e.,  $h_{\text{eff}}$  is shallower than  $h_p$ . At all locations, the  $\epsilon^2(\delta h)$  curve is qualitatively similar, and  $\delta h_{\text{opt}}$  is always negative, varying from -1.6 to -0.1 m. Note,  $\delta h_{\text{opt}}$  is entirely based on the pressure and wave buoy observations and does not consider the bathymetry.



FIG. 5. Mean squared error  $\epsilon^2$  (7) between  $H_p^2$  and  $H_{sp}^2$  versus  $\delta h$  at location S3. The minimum of  $\epsilon^2$  gives the best correction factor ( $\delta h_{opt}$ ).

Although we find negative optimal depth correction at all locations,  $\delta h_{\text{opt}}$  alone does not inform what bathymetry smoothing length scale is appropriate for computing  $h_{\text{eff}}$ .

# 288 b. Effective depth from bathymetry

Since we have the bathymetry around the Smart Mooring array, the bathymetry can be smoothed 289 to find which length scale yields a depth correction consistent with  $\delta h_{opt}$ . We first quantify the 290 rough rocky bathymetry depth statistics near the pressure sensor at S3 (Fig. 1c) from the ungridded 291 bathymetry with its probability density function (pdf, Fig 6). Within a radius of 10 m (the nominal 292 water depth), large depth variability occurs with max-min range of 5 m, and the 1/3 to 2/3 quantile 293 range is 1.7 m (Fig. 6). The pressure-sensor measured time-average depth  $\bar{h}_p = 9.7$  m is towards 294 the deeper tail of the pdf, deeper than the mean and median depths of  $\approx 8.9$  m (a difference of 295 0.8 m) and consistent with the pressure sensor located in a bathymetric low. 296

We compute depth statistics for radii between  $2 \le r \le 20$  m at 1 m intervals to find an appropriate horizontal smoothing scale to estimate  $h_{\text{eff}}$ . At every *r*, the depth correction is given by

$$\delta h(r) \equiv [h](r) - h_p, \tag{8}$$

where [h](r) is either the mean or median water depth within a distance *r* from the pressure sensor (e.g. circle with r = 10 m is shown in Fig. 1c). In general, each location has an optimal smoothing scale resulting in a *r* and a  $\delta h$  that matches  $\delta h_{opt}$ . For a simple depth correction that can be applied to any sensor, a single length scale was determined by minimizing the depth correction error, i.e.,

$$\mathcal{E}^{2}(r) = \langle \left(\delta h(r) - \delta h_{\text{opt}}\right)^{2} \rangle, \tag{9}$$



FIG. 6. Probability density function of water depths *h* within a circul with radius r = 10 m (circle in Fig. 1c) centered on the pressure sensor location. The bathymetry statistics mean depth (red), median depth (solid black), 1/3 and the 2/3 quantiles (dashed black lines) are shown as is the time-mean pressure sensor estimated depth  $\bar{h}_p$ (yellow).



FIG. 7. Average bathymetric correction error  $\mathcal{E}^2$  (9) versus *r* based on mean (blue) and median (orange) water depths.

where  $\langle \cdot \rangle$  is an average across eight locations. The minimum of  $\mathcal{E}^2$  gives the optimal smoothing horizontal length scale  $\hat{r}$  and we define  $\delta h_{\text{bathy}} \equiv \delta h(\hat{r})$ .

The mean squared errors  $\mathcal{E}^2$  have well-defined global minima at  $\hat{r} = 11$  m for the mean and  $\hat{r} = 13$  m for the median (Fig. 7). The median at  $\hat{r} = 13$  m has  $\mathcal{E}^2$  that is 25% reduced from the mean at  $\hat{r} = 11$  m, suggesting that the median bathymetry at  $\hat{r} = 13$  m is the appropriate smoothing scale at this water depth. We calculate  $\delta h_{\text{bathy}}$  at all locations using the median bathymetry at  $\hat{r} = 13$  m. As expected, all locations have  $\delta h_{\text{bathy}} < 0$ , indicating that pressure sensors were in relative bathymetric lows. Five locations (S1, S2, S3, S7, S8) had rougher bathymetry and deeper bathymetric lows  $-1.5 \le \delta h_{\text{bathy}} \le -1$  m, whereas  $\delta h_{\text{bathy}} > -0.7$  m indicates a smoother bottom at the other three locations (S4, S5, S6).



FIG. 8. Optimal depth correction  $-\delta h_{opt}$  versus smoothed bathymetry depth correction  $-\delta h_{bathy}$  using median depth with  $\hat{r} = 13$  m. The dashed line represents the 1-to-1 line. The horizontal bars represent the 1/3 to 2/3 bathymetry quantile range. The vertical error bars represent the uncertainty in the  $\delta h_{opt}$  estimate.

# $_{322}$ c. Accuracy of transfer function at $h_{eff}$

The bathymetric corrections  $\delta h_{\text{bathy}}$  using a single smoothing scale  $\hat{r} = 13$  m are consistent with 323  $\delta h_{opt}$  (Fig. 8). In terms of magnitude, the corrections qualitatively have two groupings. The first 324 grouping (S4, S5, S6) has smaller ( $\leq 0.7$  m) depth corrections  $\delta h_{\text{bathy}}$ , whereas the second grouping 325 (S1, S2, S3, S7, S8) has larger 1–1.5 m corrections (Fig. 8). These two groupings are separated by 326 the  $\delta h_{opt}$  error bars and the 1/3-2/3 bathymetric quantiles (vertical and horizontal bars in Fig. 8). 327 The  $\delta h_{\text{bathy}}$  is roughly proportional to  $\delta h_{\text{opt}}$  with a near-one slope. Location S8 has the largest 328 deviation from the one-to-one line with a 0.8 m difference between  $\delta h_{opt}$  and  $\delta h_{bathy}$ . The overall 329 similarity between  $\delta h_{\text{opt}}$  and  $\delta h_{\text{bathy}}$  supports the effective depth hypothesis. 330

We next explore how using a  $h_{\text{eff}}$  derived from either  $\delta h_{\text{opt}}$  or  $\delta h_{\text{bathy}}$  improves the significant wave 334 height estimates at location S3 (Fig. 9), a site with relatively large  $\delta h_{opt} = -1.5$  m and  $\delta h_{bathy} = -1$  m. 335 As in Fig. 2, the uncorrected  $H_p^2$  leads to large errors  $\epsilon_0^2 = 80 \text{ cm}^2$  and a large regression slope 336 of 1.19 (Fig. 9a). With the optimal correction  $h_{\text{eff}} = h_p + \delta h_{\text{opt}}$ ,  $H_p^2$  more closely matches  $H_{\text{sp}}^2$ 337 (Fig. 9b) with best-fit slope near one and small  $\epsilon^2 = 20 \text{ cm}^2$ . With the bathymetric correction 338  $h_{\text{eff}} = h_p + \delta h_{\text{bathy}}, H_p^2$  is also much closer to  $H_{\text{sp}}^2$  than for the uncorrected. The error  $\epsilon^2 = 26 \text{ cm}^2$ , 339 is slightly elevated from that of  $\delta h_{opt}$  and reduced nearly 70% relative to the uncorrected. The 340 best-fit slope is 1.08, indicating a nearly 60% reduction in the bias. 341



FIG. 9.  $H_p^2$  versus  $H_{sp}^2$  at location S3, where the transfer function is computed at (a)  $\bar{h}_p$ , (b)  $h_{eff} = \bar{h}_p + \delta h_{opt}$ , or (c)  $h_{eff} = \bar{h}_p + \delta h_{bathy}$ . The bathymetric correction  $\delta h$ , the mean squared error (7), and the linear regression slope are shown in each panel.

<sup>346</sup> Using either the optimal or bathymetrically-smoothed  $h_{eff}$  ( $< h_p$ ) reduces  $H_p^2$  to be closer to  $H_{sp}^2$ . <sup>347</sup> We next examine the effect of the two depth corrections in frequency space using the time-averaged <sup>348</sup> wave spectra  $S_\eta$  at S3 (Fig. 10). As in Fig. 3, the uncorrected  $\overline{K^2(h_p)} S_p$  overpredicts the Spotter  $\overline{S_\eta}$ <sup>349</sup> for 0.1 < f < 0.2 Hz (or 0.67 <  $k\bar{h}_p < 1.67$ , black curve Fig. 10), with ratio between their sea-band <sup>350</sup> integrated spectra of 1.17. With  $\delta h_{opt} = -1.5$  m, the spectra  $\overline{K^2(h_p + \delta h_{opt})} S_p$  is similar to the <sup>351</sup> Spotter  $\overline{S_\eta}$  across the 0.1 < f < 0.2 Hz band (green curve, Fig. 10) with ratio of their integrated <sup>352</sup> spectra of 1.00, consistent with the changes in best-fit slope (Fig. 9b). The difference between



FIG. 10. Time-averaged surface elevation spectra versus frequency at location S3. Similar to Fig. 3, the black-curve is Spotter-estimated  $\overline{S}_{\eta}$ . Three pressure-sensor estimated spectra are shown: for no depth correction (red,  $\overline{K^2(h_p)S_p}$ ), optimal depth correction (green,  $\overline{K^2(h_p + \delta h_{opt})S_p}$ ), and smoothed bathymetric correction (blue,  $\overline{K^2(h_p + \delta h_{bathy})S_p}$ ). The top axis shows the corresponding nondimensional  $k\bar{h}_p$  where  $\bar{h}_p = 9.7$  m.

 $\overline{K^2(h_p + \delta h_{opt}) S_p}$  and the uncorrected  $\overline{K^2(h_p) S_p}$  is small near f = 0.1 Hz and increases with f (or 353 *kh*). Even out to f = 0.3 Hz,  $\overline{K^2(h_p + \delta h_{opt})} S_p$  is much closer to  $\overline{S_n}$  than  $\overline{K^2(h_p)} S_p$ , indicating the 354 correction using  $\delta h_{\text{opt}}$  performs well beyond the frequency-band considered for significant wave 355 height. With  $\delta h_{\text{bathy}} = -1$  m, the spectra  $\overline{K^2(h_p + \delta h_{\text{bathy}}) S_p}$  is also similar to the Spotter  $\overline{S_{\eta}}$  for 356 0.1 < f < 0.2 Hz (blue curve, Fig. 10) with ratio of their sea-band integrated spectra of 1.05, also 357 consistent with the changes in best-fit slope (Fig. 9c). At lower frequencies f < 0.1 Hz, the spectra 358 using  $h_p$  or the optimal or bathymetric depth corrections result in similar time-mean spectra as kh359 is relatively small (< 0.66), resulting in small changes to  $K^2$ . 360

We next examine the significant wave height errors using the uncorrected ( $\delta h = 0$ ), optimal 361  $(\delta h = \delta h_{opt})$ , and smoothed bathymetry using  $\hat{r} = 13$  m ( $\delta h = \delta h_{bathy}$ ) across all eight locations 362 (Fig. 11). As seen in Fig. 2, the uncorrected error  $\epsilon_0^2 = \epsilon^2(0)$  varies by factor of 10, from 110 at 363 S2 to 11 cm<sup>2</sup> at S5 (red bars in Fig 11). The error with optimal correction  $\epsilon^2(\delta h_{opt})$  (green bars in 364 Fig 11) is reduced substantially (> 50%) relative to  $\epsilon_0^2$  at locations with significant  $\epsilon_0^2$ , such as the 365 S1, S2, S3, S7, and S8 grouping. At locations with weak  $\epsilon_0^2$  (S4 and S5), the optimal correction 366  $\delta h_{\text{opt}}$  is small and results in  $\epsilon^2(\delta h_{\text{opt}})$  are similar to  $\epsilon_0^2$ . At all locations but S8, the smoothed 367 bathymetry correction  $\epsilon^2(\delta h_{\text{bathy}})$  (blue bar, Fig. 11) is similar to  $\epsilon^2(\delta h_{\text{opt}})$ , indicating that using 368 the depth from the smoothed bathymetry enables accurate estimation of wave statistics. Location 369 S8 is an outlier, as  $\epsilon^2(\delta h_{\text{bathy}})$  is only slightly reduced from  $\epsilon_0^2$ . This location also had the largest 370 deviation between  $\delta h_{opt}$  and  $\delta h_{bathy}$  (dark red in Fig. 8). It is unclear why this location is an outlier. 371



FIG. 11. Mean squared error of  $H_p^2$  relative to  $H_{sp}^2$  (7) using local depth ( $\delta h = 0$ , red), the optimal depth correction ( $\delta h_{opt}$ , green) and the correction from the median bathymetry at  $\hat{r} = 13$  m ( $\delta h_{bathy}$ , blue).

### **5.** Discussion

For pressure sensors deployed in bathymetric lows, using linear theory with  $h_p$  gives rise to clear errors in significant wave height  $H_p^2$  and wave spectra. The agreement between  $\delta h_{opt}$  and  $\delta h_{bathy}$ and the error reduction between  $\epsilon_0^2$  and  $\epsilon^2(\delta h_{bathy})$  strongly supports the effective depth hypothesis that a water depth based on a spatially averaged bathymetry is the appropriate depth to use in linear theory on rough rocky bathymetry with large variability on small spatial scales. We next examine the errors between pressure- and Spotter-based wave statistics relative to a sandy, smooth inner-shelf, discuss the implications of an effective depth, and application to other regions.

### <sup>382</sup> a. Comparison to a smooth, sandy inner-shelf

Comparisons between co-located pressure sensors and Spotter wave buoys on the inner-shelf are 383 not common. In a low-sloped sandy bay in  $h \approx 7$  m depth, a pressure sensor integrated within an 384 ADCP (Acoustic Doppler Current Profiler) had good time-mean sea-surface spectra comparison in 385 the sea-swell band to a Spotter wave buoy (Lancaster et al. 2021). More recently, a range of wave 386 buoys were intercompared to a pressure sensor array in 8-m water depth on a low sloped and smooth 387 sandy beach (Collins et al. 2023). The Spotter was deployed 400 m alongshore from the pressure 388 sensor array over 3 winter months and the observed  $H_{sp}$  varied from 0.5-3 m, generally larger than 389 observed here (Fig. 2). In a 0.1-Hz wide band spanning similar kh ranges as here, the Collins 390 et al. (2023)  $\epsilon_0$  (6) is 36 cm<sup>2</sup>, which is partially attributable to process noise (e.g., true alongshore 391 variations in wave height) as the sensors were not co-located. Taking into account processes 392 noise and the larger  $H_{\rm sp}$ , the Collins et al. (2023)  $\epsilon_0 = 36 \text{ cm}^2$  is consistent with the smaller  $\epsilon$ 393 observed here at locations (S4, S5, S6, and S8) all of which except S8, had small  $\delta h_{opt}$  and  $\delta h_{bathy}$ 394 (Fig. 8). The locations with  $\epsilon_0$  much larger than that of Collins et al. (2023) (i.e., S1, S2, S3, S7) 395 had large  $\delta h_{opt}$  and  $\delta h_{bathy}$  (with magnitudes > 1 m), and the corrections significantly improved 396 the errors. The elevated mean-square-error in wave statistics for pressure sensors deployed in 397

<sup>398</sup> bathymetric lows, particularly relative to smooth sandy inner shelf, demonstrates the need for <sup>399</sup> correcting pressure-sensor based wave statistics on rough rocky bathymetries.

# <sup>400</sup> b. Implications of an effective depth

On larger scales, the ROXSI bathymetric slope is weak  $\approx 0.025$ . However, the rough rocky 401 bathymetry has large variability, with bottom slopes greater than  $0.52 (30^\circ)$ , over short horizontal 402 scales of O(1) m (Fig. 1). Even with this small-scale bathymetric variability, surface gravity 403 waves propagate coherently over rough rocky bathymetry as if there is a dispersion relationship 404 with an effective depth that is some spatial average of the bathymetry they are propagating over. 405 For pressure sensors in bathymetric lows in 10–13 m water depth, we found a smoothing length 406 scale of  $\hat{r} = 13$  m led to the largest reduction in error in the 0.1–0.2 Hz band. Therefore, high-407 resolution bathymetry mapping is required to calculate the effective depth. However, even with 408 such detailed knowledge of the bathymetry, for locations in other water depths or other frequencies, 409 the appropriate smoothing scale is unclear. Via dimensional reasoning, we argue that the relevant 410 nondimensional parameter is the ratio of the smoothing scale to the wavelength  $\hat{r}/\lambda$ . One could 411 imagine that each frequency would have its own associated smoothing scale, but here we focus on 412 a single frequency. The largest spectra corrections are near 0.2 Hz corresponding to  $k\bar{h}_p = 1.67$ 413 (Fig. 10), with corresponding wavelength  $\lambda \approx 37$  m (with  $\bar{h}_p \approx 10$  m), resulting in a ratio  $\hat{r}/\lambda \approx 1/3$ . 414 This may provide guidance for correcting pressure-sensor based wave statistics in other water 415 depths given the same kh = 1.67 cutoff. 416

The fluid dynamics of orbital velocities and pressure near the bottom of rough rocky bathymetry 417 is largely unstudied. Our results imply that the bed is effectively at  $z = -h_{\text{eff}}$  for the dispersion 418 relationship (3) and the transfer function (2) and that the wave pressure signal does not decay in the 419 vertical for  $-h_p \le z \le -h_{\text{eff}}$ . We hypothesize that the constant pressure below  $z = -h_{\text{eff}}$  is because 420 horizontal orbital velocities, which vary on horizontal scales of  $\lambda$ , are largely constrained to be 421 zero within bathymetric lows that have much shorter horizontal length scales (e.g., Fig. 1c). This 422 implies a spatially uniform but time-varying velocity potential below  $z = -h_{\text{eff}}$ , which, through 423 Bernoulli's equation, leads to a spatially uniform and time varying wave-induced pressure. Further 424 work on detailed near-bottom wave dynamics over rough rocky bathymetry is forthcoming. 425

# 426 c. Application to coral reefs

The errors in pressure-based wave measurements observed on rough rocky bathymetry may also occur in other regions with large bathymetric roughness such as coral reefs. Previous work have not addressed the accuracy of flat-bottom linear wave theory applied to coral reef pressure measurements. For known root-mean-squared bottom depth variability ( $\sigma_b$ ) in the vicinity of a

pressure sensor, the ratio  $\cosh^2(kh_p)/\cosh^2(k(h_p - \sigma_b))$  provides a rough magnitude estimate of 431 the potential overestimate in wave energy density at a single frequency from a near-bottom pressure 432 sensor in a bathymetric low. For small  $kh_p$ , the ratio is  $\approx 1$  and errors are negligible. Errors grow 433 with  $kh_p$ . Waves observed on shallow reef flats with  $h \approx 1$  m and  $\sigma_b = 15$  cm and peak periods 434 from 4-8 s, result in a 4% overestimate at 4 s (Lentz et al. 2016). Similar reef flat observations 435 with 10 s peak periods lead to no error (Sous et al. 2023). In deeper water reef observations  $\approx 7$  m, 436 errors in wave statistics are potentially larger, although the roughness in this depth was not reported 437 (Lowe et al. 2005). Over the rougher coral reefs at the Palmyra atoll, with  $0.4 \le \sigma_b \le 1.3$  m, 438 observations at  $h \approx 11$  m may be as accurate as 2% for low-frequency swell, but overestimates can 439 be as large 15% for 7-s seas (Monismith et al. 2015; Rogers et al. 2016). Larger depth changes over 440 spur-and-groove formations in coral reefs can imply biases of up to 35% from pressure sensors in 441 2-m deep grooves in water depths of 10 m for mean wave period of 6 s (Acevedo-Ramirez et al. 442 2021). This suggests that analogous errors may be present in some pressure-sensor based wave 443 statistics on coral reefs. 444

### 445 **6.** Summary

We present surface gravity wave observations from eight co-located bottom-mounted pressure 446 sensors and Spotter wave buoys in 10–13 m water depth from the five-week ROXSI field experiment, 447 at a site with rough rocky bathymetry on the Monterey Peninsula. The rough bathymetry has large 448 O(1) m vertical variability on O(1-10) m horizontal scales. Pressure sensors were deployed by 449 divers in rocky bathymetric lows to enhance sensor stability in large waves. Using the pressure-450 sensor estimated water depth  $h_p$ , the pressure-based significant wave height squared consistently 451 overestimates (as large as 21%) wave buoy measurements. Some locations have large mean square 452 error  $(\epsilon_0^2)$  between pressure- and buoy-based wave height, far larger than analogous measurements 453 on a sandy, low-sloped inner-shelf. The time-mean pressure-sensor based wave spectra are elevated 454 in sea-band (0.1-0.2 Hz) relative to the Spotter. These errors are consistent with the depth  $h_p$  used in 455 the linear-theory based transformation being too large. An effective depth hypothesis is proposed, 456 where a depth based on a spatially smoothed bathymetry is the appropriate depth to use with linear 457 theory for estimating wave statistics from pressure observations. An optimal depth correction 458  $\delta h_{opt}$  is estimated by minimizing the error between significant wave heights from pressure sensors 459 and Spotter wave buoys. The optimal correction to the local depth is  $-1.6 \le \delta h_{opt} < -0.1$  m 460 across the eight locations, where the sign is consistent with pressure sensors in bathymetric lows 461 and an effective depth shallower than  $h_p$ . A bathymetry smoothing scale of  $\hat{r} = 13$  m (or 1/3 462 of wavelength of 0.2 Hz waves in 10-m depth) is found by minimizing the squared difference 463 between the smoothed bathymetry correction and the optimal bathymetry correction. The optimal 464

and smoothed-bathymetric depth corrections are similar across locations. Both corrections, using linear theory, significantly improve errors in wave statistics, particularly in locations with large  $\delta h_{\text{bathy}}$  and large errors  $\epsilon_0^2$ . This indicates that the effective depth hypothesis is valid and that a depth based on spatially averaged bathymetry is the appropriate depth to use in linear theory on rough rocky bathymetry. For application in other depths or frequencies, we argue that the smoothing length scale should be  $\approx 1/3$  of the wavelength of the higher frequency waves of interest. Similar errors to those seen here may also be present in pressure-based wave statistics on rough coral reefs.

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Data availability statement. The data presented in this paper will be made freely available upon
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